

### AWNING TRUSS DESIGN CONSIDERATIONS

An awning truss usually refers to a cantilevered truss with a collinear support configuration, e.g., attached to a vertical wall of a building. You may have seen them at the front of commercial shops providing shelter without impeding the flow of pedestrians since there are no post and beam supports at the cantilevered end. An example in housing would be mono trusses that form eaves or continuations of eave overhangs at ground floor roof level which "hangs off" an upper storey section – usually connecting to or being supported within the floor space as shown in Figure 1. This article will highlight some of the traps for the unweary estimator or designer.

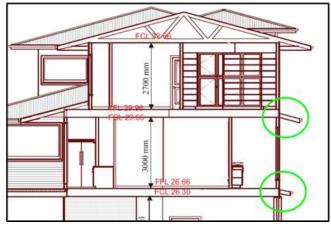


Figure 1 – Example of cantilevered eave awnings

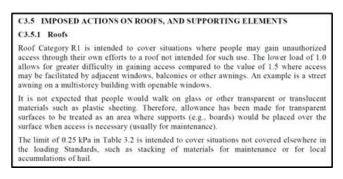
#### Trafficable or Non-trafficable roof:

Would such a small roof area be considered trafficable – especially if there's no access? For this article, we will focus on the smaller eave trusses more commonly found in residential designs but remember that awnings can certainly be much larger. Extract 1 shows Table 3.2 from our loading code AS/NZS 1170 Part 1, for roof actions (live loads) relating to different categories, activities, & specific uses of roof structures. Category "R1 Street awnings" might be obvious at first, which treats the structure like a floor, with upwards of 1.5 kPa & 1.8 kN live loads when there is direct access to the roof area.

	rpe of activity/occupancy r part of the building or structure	Specific uses	Uniformly distributed actions kPa	Concentrates actions kN
R1 Street awnings (including cladding)		Accessible from adjacent windows, roofs or balconies	1.5	1.8
		Accessible only from ground level	1.0	1.8
R2 Other roofs		(i) Structural elements	(1.8/4 + 0.12) but not less than 0.25 (see Notes 1, 2 and 3)	1.4 (see Note 3)
		(ii) Cladding providing direct support	As for structural elements	1.1
		(iii) Surfaces over which boards or ladders are required to be laid	-	0.5
NO	DTES:			12) 
1	Structural elements supporting more than 200 $m^3$ of roof area shall be designed to support 0.25 kJ <sup>a</sup> a on the 200 m <sup>2</sup> of the supported area that gives the worst effect.			
2	$\mathcal{A}$ = the plan projection of the surface area of roof supported by the member under analysis, in square metres,			
3	For structural elements in roofs of houses, the uniform distributed actions shall be 0.25 kPa and the concentrated actions shall be 1.1 kN,			

Extract 1 – Table 3.2 from AS/NZS 1170.2:2002

"R2 Other Roofs, (i) Structural Elements" apply to most of our truss designs, with Note 3 giving a concession for housing allowing smaller live loads of 0.25 kPa & 1.1 kN, typically for maintenance and construction loads. Extract 2 provides further guidance from the commentary with some examples demonstrating the intent of this clause.



#### Extract 2 – Commentary for Cl. 3.5.1 of AS/NZS 1170.2:2002

We can clearly rule out category R1 since a small eave awning is not intended as a trafficable area but there can be some arguments made that even category R2 should not apply since no one would be stupid enough to stand at the ends, right?

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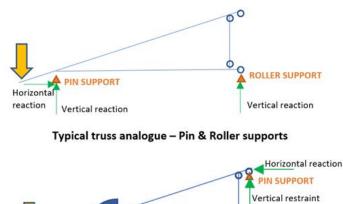
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#### MAY 2023 - ED.52

However, consider that someone must construct this area, lay the roof battens, install the roof linings etc..., and it's certainly possible that a maintenance person could set up their ladder against the gutter for access at some stage of the building life. Would you ignore the 0.25 kPa & 1.1 kN live loads with this in mind?

#### Analogue model, support conditions and connections:

When designing an awning truss using normal support assumptions (Pin–Roller or Roller–Pin) the result is an unstable system due to the supports being collinear, e.g., along a single vertical axis, where a vertical roller support cannot provide any horizontal restraint. For this reason, both supports must be modelled as Pin supports, and in doing so the connections must be designed to withstand both a horizontal and vertical reaction at each support.



Awning truss analogue - Pin & Pin supports

Horizontal reaction

Vertical reaction

Horizontal and vertical support stiffness values should be carefully considered in the design to correctly model deformations and member forces which are influenced by the connection stiffness properties. For example, connections formed with screws will have larger stiffness compared to the same connection formed with nails.

Various software packages will have differing levels of access to these properties, but the common caveat is that you will require some form of engineering support to complete these types of designs. Cici Cai - Pryda Structural Engineer

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#### **Caution for Horizontal reactions:**

Horizontal support reactions imparted on the supporting structure can have an impact, such as on lintels over large openings, which would need to be checked for bi-axial bending (bending in both axis) rather than their default design criteria of vertical loads in one axis only. Studs, jamb studs and critical studs may also require verification due to the applied reactions from awning trusses.

#### Detailing tips and tricks:

Always try and introduce a fascia beam to support the ends when possible since it is much easier to design, with the ends of fascia beam supported by a main truss or wall frame. Even if this is not possible, it is good practice to introduce some form of continuous member to tie the ends of the trusses together and help with load sharing.

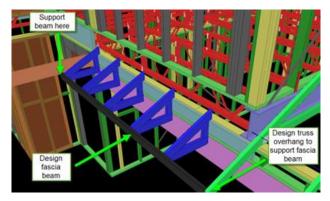


Figure 2 – Fascia beam support option where possible

Awning truss designs will need engineering assistance in one form or another, so it is best to identify them as early as possible and involve the engineering team from your software supplier to ensure you don't get caught out with inadequate designs and connections.

This edition of FTMA Tech Talk was written by Cici Cai, Structural Engineer of our Principal Parnter, Pryda.



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